

4. W14 x 109, A992, L = 36' (about both axes).

$$\frac{KL}{2}, \text{ um } z_y \Rightarrow \frac{KL}{z_y} = \frac{1 \times 36 \times 12}{3.73} = 115.82$$

K=1 pin-ended

$$4.71 \sqrt{\frac{E}{F_y}} = 4.71 \sqrt{\frac{29000}{50}} = 113.43$$

$$\Rightarrow \frac{KL}{z_y} > 4.71 \sqrt{\frac{E}{F_y}} \Rightarrow F_{cr} = 0.877 F_e$$

$$F_e = \frac{\pi^2 E}{\left(\frac{KL}{2}\right)^2} = \frac{\pi^2 29000}{(115.82)^2} = 21.34 \text{ ksi}$$

$$\Rightarrow F_{cr} = 0.877 \times 21.34 = 18.72 \text{ ksi}$$

$$\Rightarrow P_n = F_{cr} \cdot A = 18.72 \times 32 = 599 \text{ ok K}$$

$$\left(\frac{KL}{2} > 4.71 \sqrt{\frac{E}{F_y}} \Rightarrow \text{Elastic buckling} \right)$$

$$\underline{\text{LRFD}} \Rightarrow \phi P_n = 0.9 \times 599.04 = 539.14 \text{ K}$$

$$\underline{\text{ASD}} \Rightarrow \frac{P_n}{\Omega} = \frac{599.04}{1.67} = 358.71 \text{ K}$$

using λ_c , the problem is solved as follows:

$$\lambda_c = \frac{KL}{\pi r} \sqrt{\frac{F_y}{E}} = \frac{36 \times 12}{\pi \times 3.73} \sqrt{\frac{50}{29000}} = 1.531 > 1.5$$

\Rightarrow elastic

$$\Rightarrow F_{cr} = \frac{0.877}{\lambda_c^2} \cdot F_y = \frac{0.877}{(1.531)^2} \times 50 = 18.71 \text{ Ksi}$$

$$\Rightarrow P_n = F_{cr} \cdot A = 18.71 \times 32 = 598.72 \text{ K}$$

6. W12x45, A992, $KL_y = 20'$, $KL_x = 40'$

$$r_x = 5.15, \quad r_y = 1.95, \quad A = 13.1$$

$$\Rightarrow \frac{KL_y}{r_y} = \frac{20 \times 12}{1.95} = \underline{123.08} \quad \leftarrow \text{controls}$$

$$\frac{KL_x}{r_x} = \frac{40 \times 12}{5.15} = 93.20$$

$$4.71 \sqrt{\frac{E}{F_y}} = 113.43 \Rightarrow \frac{KL_y}{r_y} > 4.71 \sqrt{\frac{E}{F_y}} \Rightarrow \text{Elastic}$$

$$\Rightarrow F_{CR} = 0.877 F_e$$

$$F_e = \frac{\pi^2 E}{\left(\frac{KL_y}{r_y}\right)^2} = \frac{\pi^2 \times 29000}{(123.08)^2} = 18.89 \text{ ksi}$$

$$\Rightarrow F_{CR} = 0.877 \times F_e = 16.57 \text{ ksi}$$

$$\Rightarrow P_n = F_{CR} \cdot A = 16.57 \times 13.1 = 217.07 \text{ K}$$

LRFD: $\phi P_n = 0.9 \times 217.07 = 195.36 \text{ K}$

ASD: $\frac{P_n}{\Omega} = \frac{217.07}{1.67} = 129.98 \text{ K}$

$$7. \quad W/8 \times 24, A992, \quad K L_y = 12.5', \quad K L_x = 28'$$

$$r_x = 3.42, \quad r_y = 1.61, \quad A = 7.08$$

$$\frac{K L_y}{r_y} = \frac{12.5 \times 12}{1.61} = 93.17$$

$$\frac{K L_x}{r_x} = \frac{28 \times 12}{3.42} = 98.25 \quad \leftarrow \text{controls}$$

$$\frac{K L_x}{r_x} < 4.71 \sqrt{\frac{E}{F_y}} = 113.43 \quad \rightarrow \text{Inelastic}$$

$$\Rightarrow F_{cr} = \left(0.658^{\frac{F_y}{E}} \right) F_y$$

$$\bar{F}_e = \frac{\pi^2 E}{\left(\frac{K L_x}{r_x} \right)^2} = \frac{\pi^2 29000}{(98.25)^2} = 29.65 \text{ ksi}$$

$$\Rightarrow F_{cr} = \left(0.658^{\frac{50}{29.65}} \right) 50 = 24.69 \text{ ksi}$$

$$\Rightarrow P_m = F_{cr} A = 24.69 \times 7.08 = 174.81 \text{ k}$$

$$\underline{\text{LRFD:}} \quad \phi P_m = 0.9 \times 174.81 = 157.33 \text{ k}$$

$$\underline{\text{ASD:}} \quad \frac{P_m}{\Omega} = \frac{174.81}{1.67} = 104.68 \text{ k}$$

3. W 8x40 Col

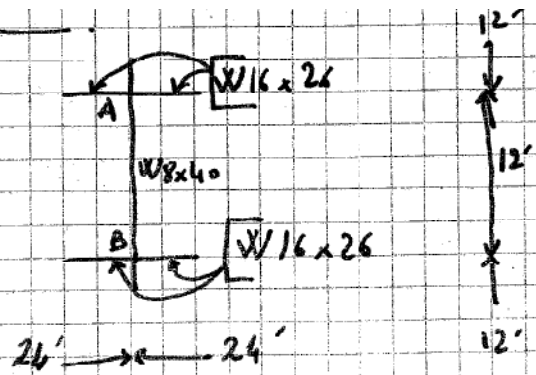
W 16x26 beams

Beam: $I_x = 301$

Col: $I_x = 146$

$I_y = 49.1$

$r_x/r_y = 1.73$



Braced frame: Sidesway inhibited

i) weak axis col:

$$G_A = \frac{\sum (E/L)_c}{\sum (E/L)_B} = \frac{2 \left(\frac{491}{12} \right)}{2 \left(\frac{301}{24} \right)} = 0.326$$

$$G_B = G_A = 0.326$$

$$\Rightarrow K \approx 0.64$$

$$\Rightarrow KL = 0.64 \times 12 = 7.68'$$

(using the eqn to find K, with $G_A = G_B$)

$$K = \frac{G+0.4}{G+0.8} = \frac{0.326+0.4}{0.326+0.8} = 0.645$$

We used $K=0.64$)

From table 4-1 in the manual,

LRFD:

$$KL=7' \rightarrow \phi P_n = 467 \text{ K}$$

$$KL=8' \rightarrow \phi P_n = 449 \text{ K}$$

$$\Rightarrow KL = 7.68' \rightarrow \phi P_n = 467 - (0.68 \times [467 - 449]) \\ = 454.76 \text{ K}$$

ASD: $KL=7' \rightarrow \frac{P_n}{\Omega} = 310$

$$KL=8' \rightarrow \frac{P_n}{\Omega} = 299$$

$$KL=7.68' = 310 - (0.68 \times [310 - 299]) \\ = 302.52 \text{ K}$$

ii) STRONG AXIS

$$G_A = G_B = \frac{E(I/L)_C}{E(I/L)_B} = \frac{2 \left(\frac{146}{12} \right)}{2 \left(\frac{301}{24} \right)} = 0.97$$

From Alignment chart $K = 0.77$

$$\text{From EQUATION } \left(\frac{G+0.4}{G+0.8} \right) = 0.774$$

$$K_L = 12(0.77) = 9.24'$$

SINCE COLUMN IS BUCKLING ABOUT STRONG AXIS, MUST ACCOUNT FOR r_x

$$K_{\text{Left}} = \frac{K_L r_x}{r_y} = \frac{9.24}{1.73} = 5.34 \text{ ft}$$

LRFD

$$K_L = 0 \quad \phi P_n = 528 \text{ k}$$

$$K_L = 6 \text{ ft} \quad \phi P_n = 482 \text{ k}$$

$$\text{for } K_L = 5.34 \text{ ft} \quad \phi P_n = \underline{\underline{487 \text{ k}}}$$

ASD

$$K_L = 0 \quad P_n / \Omega = 351 \text{ k}$$

$$K_L = 6 \text{ ft} \quad P_n / \Omega = 321 \text{ k}$$

$$\text{for } K_L = 5.34 \text{ ft} \quad P_n / \Omega = \underline{\underline{324 \text{ k}}}$$