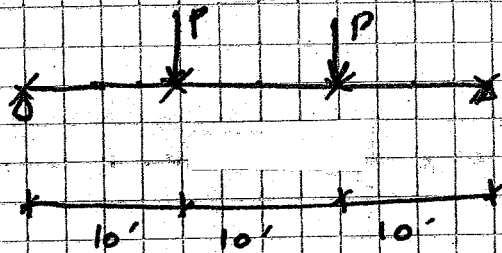


17. $P_D = 12$

$P_L = 18$



Assume $C_b = 1.0$

a) LRF: $P_u = 1.2(12) + 1.6(18) = 43.2 \text{ K}$

$\Rightarrow \Omega_u = P_u \times 10 = 432 \text{ ft}\cdot\text{K}$

try: W 21 x 55:

$\phi \Omega_n = 473$, $L_p = 6.11 \text{ ft}$ $BF = 16.3$

$L_2 = 17.4 \text{ ft}$

$L_b = 10' > L_p$ $L_b = 10 < L_2$

$\Rightarrow \phi \Omega_n = \phi \Omega_p - BF(L_b - L_p) \leq \phi \Omega_p$

$= 473 - 16.3(10 - 6.11) \leq 473$

$= 409.6 \text{ ft}\cdot\text{K}$

$\Omega_u = 432 \text{ ft} > 409.6 \text{ ft} \Rightarrow \text{W } 21 \times 55 \text{ Not good}$

try W 24 x 55

$\phi \Omega_p = 503$, $L_p = 4.73 > L_b$ $BF = 22.2$

$L_2 = 13.9 < L_b$

$\Rightarrow \phi \Omega_n = \phi \Omega_p - BF(L_b - L_p)$

$= 503 - 22.2(10 - 4.73) = 386.0 \text{ ft}\cdot\text{K} < \Omega_u = 409$

$\Rightarrow \text{Not good}$

Try W 21x62: $\phi P_p = 540$ $L_p = 6.25 < L_b$ $BF = 17.4$
 $L_2 = 18.1 > L_b$

$\Rightarrow \phi P_n = \phi P_p - BF(L_b - L_p)$
 $= 540 - 17.4(10 - 6.25) = 474.75 \text{ ft.k} > 432 \text{ ft.k}$
 $\Rightarrow \text{OK}$

b) ASD: $P_a = 12 + 18 = 30 \text{ k}$

$P_a = 30 \times 10 = 300 \text{ ft.k}$

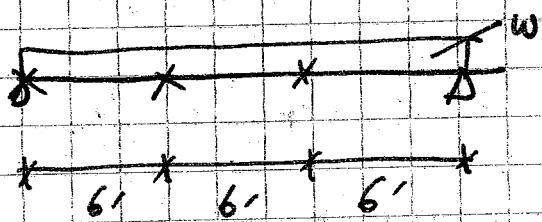
Try W 24x55: $\frac{P_p}{\Omega} = 334$, $L_p = 4.73$ $BF = 14.8$
 $L_2 = 13.9$

$\frac{P_n}{\Omega} = \frac{P_p}{\Omega} - BF(L_b - L_p)$
 $= 334 - 14.8(10 - 4.73) = 256 < 300 \Rightarrow \text{Not good}$

Try W 21x62: $\frac{P_p}{\Omega} = 359$ $L_p = 6.25$ $BF = 11.6$
 $L_2 = 18.1$

$\frac{P_n}{\Omega} = \frac{P_p}{\Omega} - BF(L_b - L_p)$
 $= 359 - 11.6(10 - 6.25) = 315.5 > 300 \Rightarrow \text{OK}$

18.



$w_D = 1.4 \text{ k/ft}$
 $w_L = 2.3 \text{ k/ft}$

$G_0 = 1.45 \quad 1.01 \quad 1.45 \quad (\text{TABLE 3-1})$

a) LRFD: $w_u = 1.2(1.4) + 1.6(2.3) = 5.36 \text{ k/ft}$

$$\Rightarrow \Pi_u = 5.36 \times \frac{18^2}{8} = 217.1 \text{ ft.k}$$

$$L_b = 6'$$

Try W 18x35, $\phi \Pi_p = 249$ $L_p = 4.31 < L_b$ $BF = 12.1$

$$L_2 = 12.4 > L_b$$

$$\begin{aligned} \phi \Pi_u &= C_b \left[\phi \Pi_p - BF(L_b - L_p) \right] \leq \phi \Pi_p \\ &= 1.01 \left[249 - 12.1(6 - 4.31) \right] \leq 249 \\ &= 230.84 \text{ ft.k} \end{aligned}$$

$$\phi \Pi_u > \Pi_u = 217.1 \Rightarrow \text{OK}$$

(we can ~~find~~^{go} in table 3-10, with $L_b = 6'$
 \Rightarrow W 18x35 is good)

b) ASD: $w_a = 1.4 + 2.3 = 3.7 \text{ k/ft}$

$$\Pi_a = 3.7 \times \frac{18^2}{8} = 149.85 \text{ ft.k}$$

Try W 18x35, $\frac{\Pi_p}{\Omega} = 166$ $L_p = 4.31 < L_b$ $BF = 8.07$

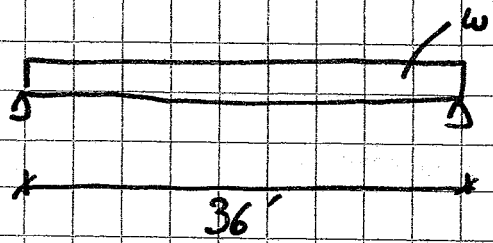
$$L_2 = 12.4 > L_b$$

$$\begin{aligned} \frac{\Pi_u}{\Omega} &= C_b \left[\frac{\Pi_p}{\Omega} - BF(L_b - L_p) \right] \leq \frac{\Pi_p}{\Omega} \\ &= 1.01 \left[166 - 8.07(6 - 4.31) \right] \leq 166 \end{aligned}$$

$$= 153.89 \text{ ft.k} > 149.85 \text{ ft.k} \Rightarrow \text{OK}$$

from table 3-10, W 18x35 is OK.

19. W18x60, A992,



When to put lateral supports to sustain

$$P_u = 435 \text{ ft.k}$$

$$P_u = 280 \text{ ft.k} \quad ?$$

a) LRFD, $P_u = 435 \text{ ft.k}$.

$$W18x60 : \phi P_p = 461 \text{ ft.k} \quad L_p = 5.93' \quad BF = 14.5$$

$$\phi P_2 = 284 \text{ ft.k} \quad L_2 = 18.2'$$

$P_u < \phi P_p$ but $P_u > \phi P_2$

$$\Rightarrow P_u \geq \phi P_n = C_b [\phi P_p - BF (L_b - L_p)] \leq \phi P_p$$

Let's assume $C_b = 1.0$

$$\Rightarrow P_u = \phi P_n = \phi P_p - BF (L_b - L_p)$$

$$\Rightarrow 435 = 461 - 14.5 (L_b - 5.93)$$

$$\Rightarrow 14.5 (L_b - 5.93) = 461 - 435 = 26$$

$$\Rightarrow L_b = \frac{26}{14.5} + 5.93 = 7.72'$$

\Rightarrow lateral support @ fifth of the length $\Rightarrow L_b = 7.2'$

& $C_b = 1.0$.

$$\Rightarrow \phi P_n = 461 - 14.5 (7.2 - 5.93) = 442.6 > 435 \text{ ft.k}$$

\Rightarrow OK

b) ASD: $\phi_a = 290 \text{ ft}\cdot\text{K}$

W18x60: $\frac{P_p}{\Omega} = 307 \text{ ft}\cdot\text{K}$; $\frac{P_2}{\Omega} = 189 \text{ ft}\cdot\text{K}$

$$\Rightarrow \frac{P_2}{\Omega} < \phi_a < \frac{P_p}{\Omega} \Rightarrow L_p < L_b < L_2$$

$$L_p = 5.93, \quad L_2 = 18.2, \quad BF = 9.64$$

Let's assume $C_b = 1.0$

$$\Rightarrow \phi_a \leq \frac{P_m}{\Omega} = C_b \left[\frac{P_p}{\Omega} - BF(L_b - L_p) \right]$$

$$\Rightarrow 290 = 307 - 9.64(L_b - 5.93)$$

$$\Rightarrow L_b = 7.7'$$

\Rightarrow Lateral support @ fifth L, $L_b = 7.2'$, $C_b = 1.0$

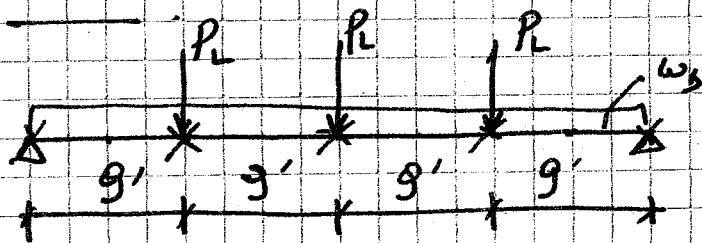
$$\Rightarrow \phi_a \leq \frac{P_m}{\Omega} = 1.0 \left[307 - 9.64(7.2 - 5.93) \right]$$

$$\Rightarrow 290 < 294.76 \text{ ft}\cdot\text{K} \Rightarrow \text{OK}$$

20.

$$w_D = 1.7 \text{ K/ft}$$

$$P_L = 15 \text{ K}$$



$C_b = 1.11$ (for concentrated loads, approximately for actual case)

a) LRFD: $w_u = 1.2(1.7) = 2.04 \text{ K/ft}$

$$P_u = 1.6(15) = 24 \text{ K}$$

$$\begin{aligned}\Rightarrow \phi M_u &= w_u \frac{L^2}{8} + P_u \cdot 9 + P_u \frac{L}{4} \\ &= 2.04 \times \frac{36^2}{8} + 24 \times 9 + 24 \times \frac{36}{4} \\ &= 330.48 + 216 + 216 \\ &= 762.48 \text{ ft.K}\end{aligned}$$

$$L_b = 9'$$

from table 3-10, try W24x84

$$\Rightarrow \phi_b M_g = \frac{84}{1000} \times \frac{36^2}{8} = 13.61 \text{ ft.K}$$

$$\Rightarrow \phi M_u = 762.48 + 1.2 (13.61) = 778.81 \text{ ft.K}$$

$$\begin{aligned}\phi M_n &= C_b [\phi M_p - BF(L_b - L_p)] \leq \phi M_p \\ &= 1.11 [840 - 24.3(9 - 6.89)] \leq 840 \\ &= 875.43 \neq 840 \\ \phi M_n &= 840 \text{ ft.K} \Rightarrow \underline{\text{OK}}\end{aligned}$$

THIS CHECK NOT
NECESSARY SINCE
W24x84 WORKS
WITHOUT C_b

Check deflection under live load:

$$\delta = \frac{Pl^3}{48EI} + \frac{Pa}{24EI} (3l^2 - 4a^2) \leq \frac{36 \times 12}{360} = 1.2''$$

$$I = 2370 \text{ in}^4, E = 29000 \text{ ksi}$$

$$\begin{aligned}\Rightarrow \delta &= \frac{15 \times (36 \times 12)^3}{48 \times 29000 \times 2370} + \frac{15 \times 9 \times 12}{24 \times 29000 \times 2370} (3 \times (36 \times 12)^2 - 4 \times (9 \times 12)^2) \\ &= 0.367 + 0.504 = 0.871'' < 1.2'' \therefore \text{OK}\end{aligned}$$

b) ASD

$$W_a = 1.7 \text{ k/ft}$$

$$P_a = 15 \text{ k}$$

$$M_a = 1.7 \times \frac{36^2}{8} + 15 \times 9 + 15 \times \frac{36}{4} = 545.4 \text{ ft-k}$$

TABLE 3-10 $h_b = 9'$, W27x84

self weight

$$M = \frac{84}{1000} \times \frac{36^2}{8} = 13.6 \text{ ft-k}$$

$$M_a = 545.4 + 13.6 = 559 \text{ ft-k}$$

W27x84 STILL GOOD

$$I = 2850 \text{ in}^4$$

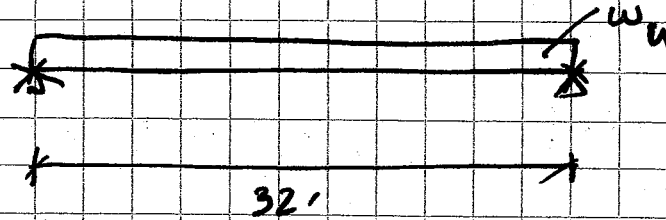
$$\delta = \frac{15 \times (36 \times 12)^3}{48 \times 29000 \times 2850} + \frac{15 \times 9 \times 12}{24 \times 29000 \times 2850} \left(3 \times (36 \times 12)^3 + 4 \times (9 \times 12)^3 \right)$$

$$= 0.305 + 0.419 = 0.724'' < 1.2'' \therefore \text{OK}$$

21.

$$w_D = 2.3 \text{ k/ft}$$

$$w_L = 3.1 \text{ k/ft}$$



a) LRFD,

$$w_u = 1.2(2.3) + 1.6(3.1) = 7.72 \text{ k/ft}$$

$$C_b = 1.14$$

$$M_u = 7.72 \times \frac{32^2}{8} = 988.16 \text{ ft.k}$$

$$L_b = 32'$$

Enter table 3-10 with $\frac{M_u}{C_b} = \frac{988.16}{1.14} = 867 \text{ ft.k}$

try W 24 x 146

$$M_g = \frac{146}{1000} \times \frac{32^2}{8} = 18.69 \text{ ft.k}$$

$$\Rightarrow M_u = 988.16 + 1.2 \times (18.69) = 1010.6 \text{ ft.k}$$

W 24 x 146 : $\phi N_p = 1570$, $L_p = 10.6$ $L_b = 32$

$$L_2 = 33.7 \quad BF = 25.8$$

$$\begin{aligned} \phi N_n &= C_b [\phi N_p - BF(L_b - L_p)] \leq \phi N_p \\ &= 1.14 [1570 - 25.8(32 - 10.6)] \leq 1570 \\ &= 1.14 \times 1017.88 = 1160.38 < 1570 \end{aligned}$$

$$\Rightarrow M_u < \phi N_n \Rightarrow \text{OK}$$

Check Shear:

$$V_u = 7.72 \times \frac{32}{2} + \frac{146}{1000} \times \frac{32}{2} \times 1.2 = 126.32 \text{ K}$$

$$\phi V_n = 482 \text{ K} > V_u \Rightarrow \underline{\text{OK}}$$

Check deflection:

$$\delta_L = \frac{5}{384} \frac{w_u L^4}{EI} \leq \frac{1}{360} \times L = 1.07''$$

$$= \frac{5}{384} \times \frac{3.1 \times 32^4 \times 12}{29000 \times 4580} = 0.55'' < 1.07'' \Rightarrow \underline{\text{OK}}$$

6) ASD:

$$w_a = 2.3 + 3.1 = 5.4 \text{ K/ft}$$

$$\eta_a = 5.4 \times \frac{32^2}{8} = 691.2 \text{ ft.K}$$

Enter Table 3-10 with $\frac{\eta_a}{C_b} = \frac{691.2}{1.14} = 606.32 \text{ ft.K}$

$$L_b = 32' \Rightarrow \text{W24} \times 146$$

Try W24 x 146

$$\eta_g = 18.63 \text{ ft.K}$$

$$\Rightarrow \eta_a = 691.2 + 18.63 = \cancel{709.8} 709.9 \text{ ft.K}$$

$$\frac{\eta_m}{\Omega} = 1.14 \left[\frac{\eta_p}{\Omega} - BF(L_b - L_p) \right] \leq \frac{\eta_p}{\Omega}$$

$$= 1.14 \left[1040 - 17.1(32 - 10.6) \right] = 768.43 \leq 1040$$

$$\phi_a = 709.9 \text{ ft}\cdot\text{K} < \frac{\phi_m}{\Omega} = 768.43 \text{ ft}\cdot\text{K} \Rightarrow \text{OK}$$

Check Shear:

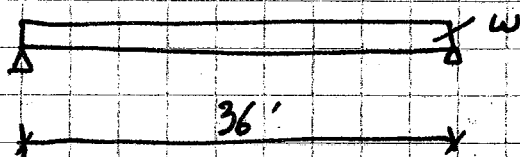
$$V_a = 5.4 \times \frac{32}{2} + \frac{146}{1000} \times \frac{32}{2} = 88.74 \text{ K}$$

$$\frac{V_m}{\Omega} = 322 \text{ K} \Rightarrow V_a < \frac{V_m}{\Omega} \Rightarrow \text{OK}$$

Deflection is OK SAME AS FOR LRFD.

$$22. w_D = 3.4 \text{ K/ft}$$

$$w_L = 2.4 \text{ K/ft}$$



$$C_b = 1.0$$

$$L_b = 0$$

a) LRFD:

$$w_u = 1.2(3.4) + 1.6(2.4) = 7.92 \text{ K/ft}$$

$$\Rightarrow \phi_u = 7.92 \times \frac{36^2}{8} = 1283.04 \text{ ft}\cdot\text{K}$$

Table 3-2 \Rightarrow W 30 x 108

$$\text{Try W 30 x 108} \Rightarrow \phi \phi_p = 1300 \text{ ft}\cdot\text{K}, \phi \phi_v = 488 \text{ K}$$

$$\phi_g = \frac{108}{1000} \times \frac{36^2}{8} = 17.5 \text{ ft}\cdot\text{K}$$

$$V_g = \frac{108}{1000} \times \frac{36}{2} = 1.94 \text{ K}$$

$$\Rightarrow \phi_u = 1283.04 + 1.2(17.5) = 1304.04 \text{ ft}\cdot\text{K} > 1300 \text{ ft}\cdot\text{K}$$

\Rightarrow NOT good!

⇒ try W 30 x 116

$$\eta_g = \frac{116}{1000} \times \frac{36^2}{8} = 18.79 \text{ ft}\cdot\text{K}$$

$$V_g = \frac{116}{1000} \times \frac{36}{2} = 2.09 \text{ K}$$

$$\Rightarrow \eta_u = 1283.04 + 1.2(18.79) = 1305.53 \text{ ft}\cdot\text{K} < 1420 \Rightarrow \text{OK}$$

$$V_u = 7.92 \times \frac{36}{2} + 1.2(2.09) = 145.07 \text{ K} < 503 \text{ K} \Rightarrow \text{OK}$$

Check deflection:

$$\delta_{\text{man}} = \frac{L}{360} = \frac{36 \times 12}{360} = 1.2''$$

$$\delta_L = \frac{5}{384} \times \frac{2.4 \times 36^4 \times 12^3}{29000 \times 4930} = 0.63'' < 1.2'' \Rightarrow \text{OK}$$

b) ASD:

$$\eta_a = (3.4 + 2.4) \frac{36^2}{8} = 939.6 \text{ ft}\cdot\text{K}$$

$$V_a = (3.4 + 2.4) \frac{36}{2} = 104.4 \text{ K}$$

Table 3-2, ⇒ W 30 x 116

try W 30 x 116

$$\eta_a = 939.6 + 18.79 = 958.4 \text{ ft}\cdot\text{K} > \frac{\eta_p}{2} = 943 \text{ ft}\cdot\text{K}$$